

FOLLETT CORPORATION

12CI425A DISPENSER

DES. **J. ROBERSON**

JOB NO. **11-1420**

DATE **5/9/14**

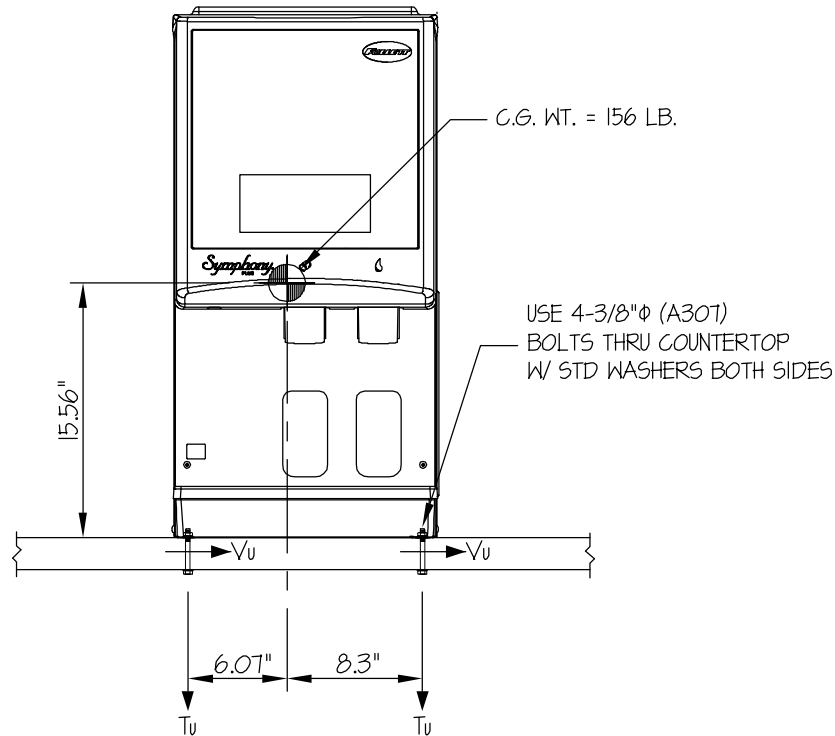
SHEET

1

OF **2** SHEETS

SEISMIC ANCHORAGE

COUNTERTOP MOUNTED



$T_u = 196 \text{ LB/BOLT (MAX)}$
 $V_u = 81 \text{ LB/BOLT (MAX)}$

FRONT ELEVATION

NOTES:

- FORCES ARE DETERMINED PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10 STRENGTH DESIGN IS USED.

HORIZONTAL FORCE (E_h) = $1.80 W_p$ ($S_{ds} = 2.5, a_p = 1.0, I_p = 1.5, R_p = 2.5, z/h \leq 1$)

VERTICAL FORCE (E_v) = $0.50 W_p$

- CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THESE CALCULATIONS ENCOMPASS ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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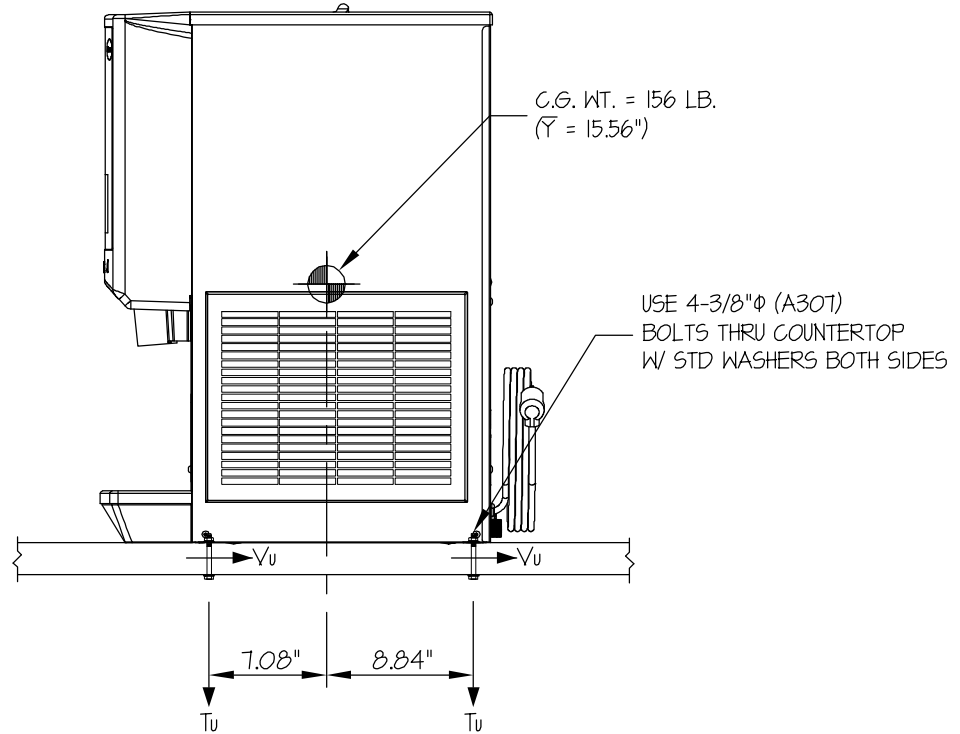
SHEET

2

OF **2** SHEETS

SEISMIC ANCHORAGE

COUNTERTOP MOUNTED



SIDE ELEVATION

LOADS: PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10.

(STRENGTH DESIGN IS USED) ($S_Ds = 2.5$, $a_p = 1.0$, $I_p = 1.5$, $R_p = 2.5$, $z/h \leq 1$)

WEIGHT = 156 LB

HORIZONTAL FORCE (E_h) = 1.80 $W_p = 281$ LB

VERTICAL FORCE (E_v) = 0.50 $W_p = 78$ LB

BOLT FORCES:

BOLT SPEC: 3/8" phi (A307) BOLTS

$\phi T = 3589$ LB/BOLT

$\phi V = 1914$ LB/BOLT

TENSION (T)

$$T_u \text{ MAXIMUM} = \left[\frac{281\#(15.56'')(8.3'')}{1 \text{ BOLT } (15.92'')(14.37'')} \times (0.3) \right] + \frac{281\#(15.56'')(8.84'')}{1 \text{ BOLTS}(15.92'')(14.37'')} - \frac{(156\#(0.9) - 78\#)(8.3'')(8.84'')}{1 \text{ BOLT } (14.37'')(15.92'')} = 196 \text{ LB/BOLT (MAX)}$$

(HORIZ. - FRONT TO BACK) (HORIZ. - SIDE TO SIDE) (0.9WEIGHT) - E_v)

SHEAR (V)

$$V_u \text{ MAXIMUM} = \frac{281\#(8.3'')}{2 \text{ BOLTS}(14.37'')} = 81 \text{ LB/BOLT (MAX)}$$