

FOLLETT CORPORATION

110FB425 A/W DISPENSERS

DES. **J. ROBERSON**

JOB NO. **11-1420**

DATE **6/12/14**

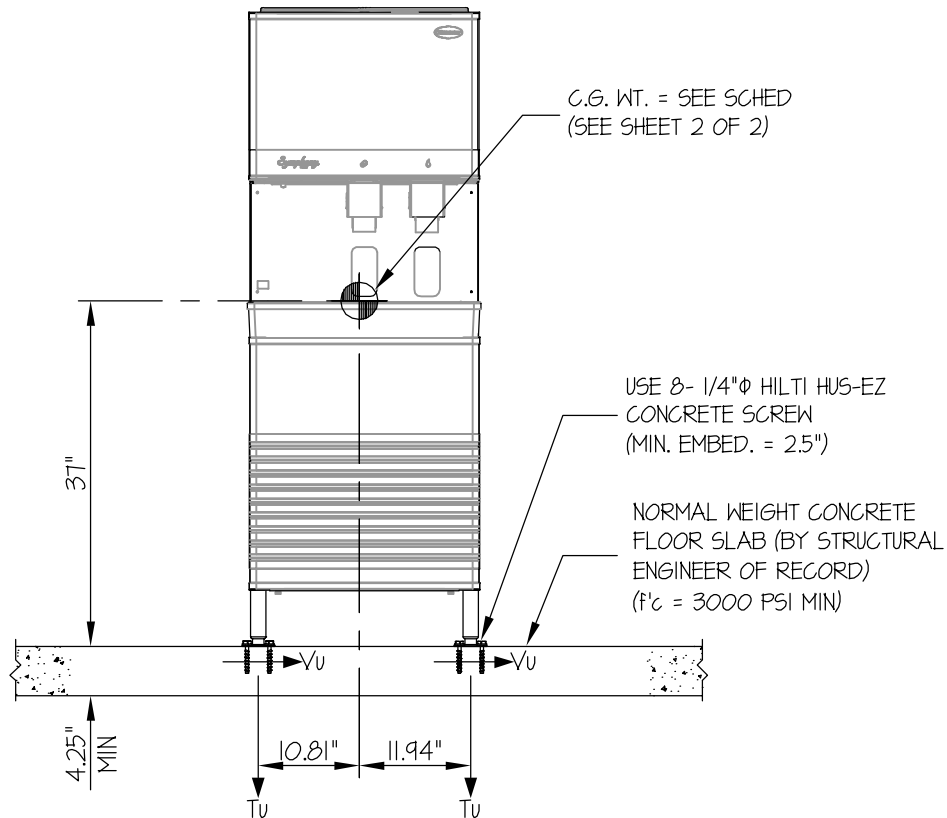
SHEET

1

OF **2** SHEETS

SEISMIC ANCHORAGE

SLAB ON GRADE



FRONT ELEVATION

NOTES:

1. FORCES ARE DETERMINED PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10 STRENGTH DESIGN IS USED. ($S_{Ds} = 2.50$, $a_p = 1.0$, $I_p = 1.5$, $R_p = 2.5$, $\Omega_0 = 2.5$, $z/h = 0$)

HORIZONTAL FORCE (E_h) = $1.125 W_p$

HORIZONTAL FORCE (E_{mh}) = $2.81 W_p$ (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (E_v) = $0.50 W_p$

- CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THESE CALCULATIONS ENCOMPASS ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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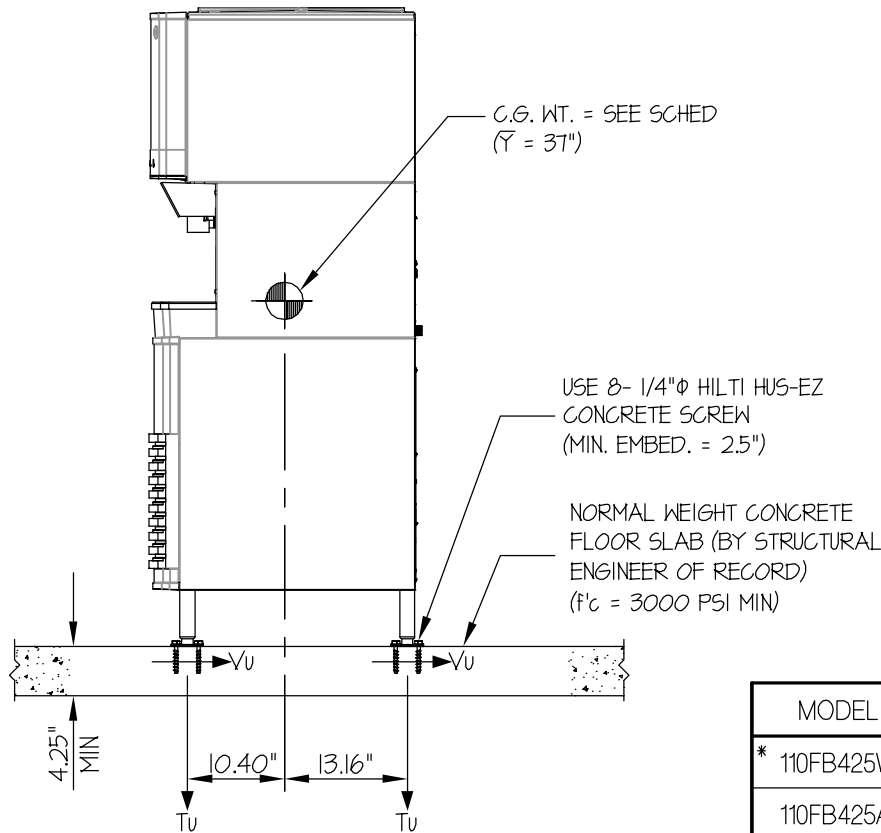
SHEET

2

OF **2** SHEETS

SEISMIC ANCHORAGE

SLAB ON GRADE



MODEL	MAX WT	T _u	V _u
* 110FB425W	360#	563#	141#
110FB425A	355#	556#	139#

* USED IN CALCULATION

SIDE ELEVATION

LOADS: PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED ($S_Ds = 2.50$, $a_p = 1.0$, $l_p = 15$, $R_p = 2.5$, $\Omega_o = 2.5$, $z/h = 0$)

WEIGHT = 360 LB

HORIZONTAL FORCE (E_{mh}) = $2.81 W_p = 1012$ LB

VERTICAL FORCE (E_v) = $0.50 W_p = 180$ LB

BOLT FORCES:

BOLT SPEC: 1/4" HILTI HUS -EZ

$\phi T = 0.75 \phi N_n = 623$ LB/BOLT (TENSION)

$\phi V = \phi v_n = 836$ LB/BOLT (SHEAR)

TENSION (T)

$$T_u \text{ MAXIMUM} = \left[\frac{1012\#(37\")(11.94\")}{2 \text{ BOLTS}(23.56\")(22.75\")} \times (0.3) \right] + \frac{1012\#(37\")(13.16\")}{2 \text{ BOLTS}(22.75\")(23.56\")} - \frac{(360\#)(0.9) - 180\#(11.94\")(13.16\")}{2 \text{ BOLTS}(22.75\")(23.56\")} = 563 \text{ LB/BOLT (MAX)}$$

(HORIZ. - FRONT TO BACK) (HORIZ. - SIDE TO SIDE) (0.9WEIGHT) - E_v)

SHEAR (V)

$$V_u \text{ MAXIMUM} = \frac{1012\#(13.16\")}{4 \text{ BOLTS}(23.56\")} = 141 \text{ LB/BOLT (MAX)}$$

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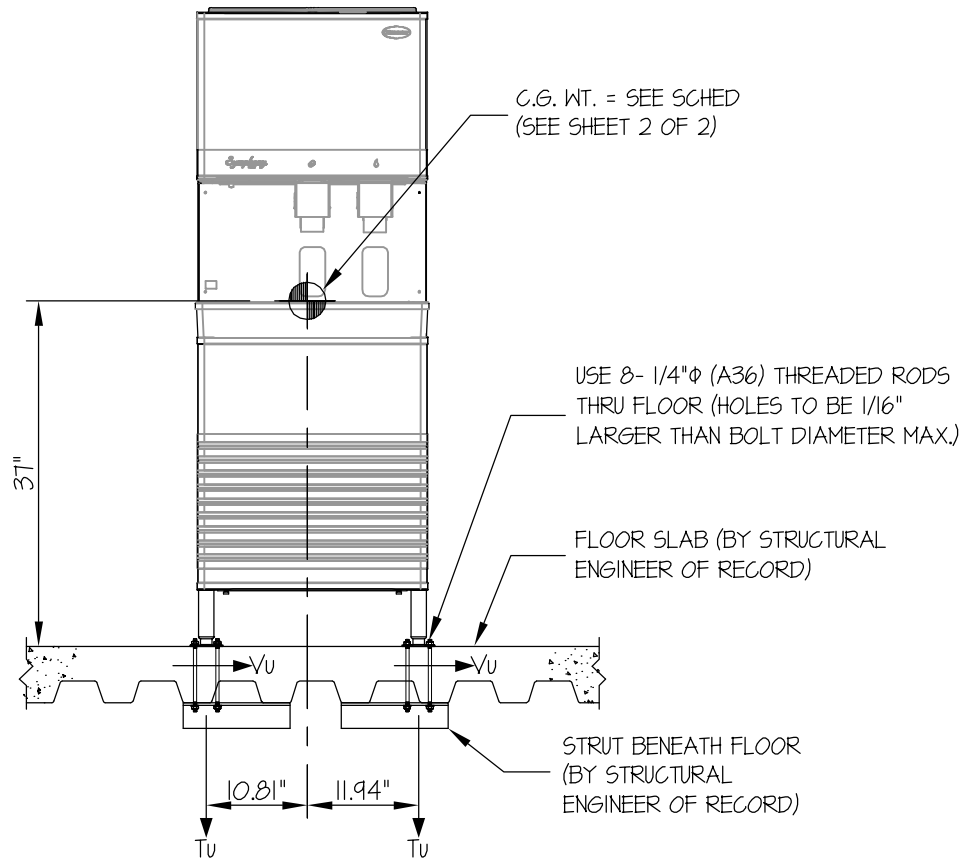
SHEET

1

OF **2** SHEETS

SEISMIC ANCHORAGE

UPPER FLOOR



FRONT ELEVATION

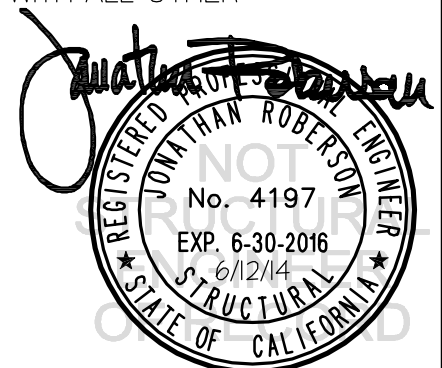
NOTES:

- FORCES ARE DETERMINED PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10 STRENGTH DESIGN IS USED. ($S_{ds} = 2.50$, $a_p = 1.0$, $I_p = 1.5$, $R_p = 2.5$, $z/h \leq 1$)

HORIZONTAL FORCE (E_h) = $1.80 W_p$

VERTICAL FORCE (E_v) = $0.50 W_p$

- CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THESE CALCULATIONS ENCOMPASS ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
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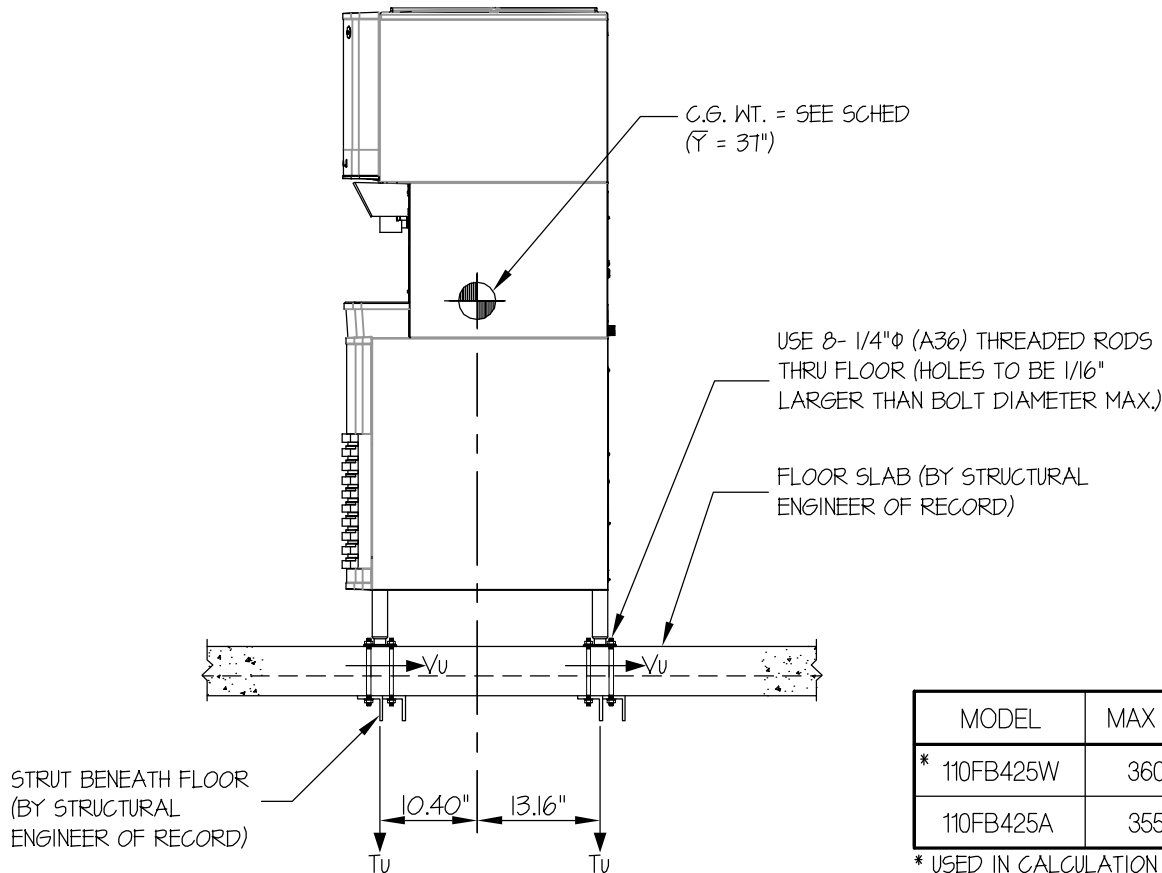
SHEET

2

OF **2** SHEETS

SEISMIC ANCHORAGE

UPPER FLOOR



MODEL	MAX WT	T _u	V _u
* 110FB425W	360#	353#	90#
110FB425A	355#	348#	89#

* USED IN CALCULATION

LOADS: PER 2013 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED ($S_{ds} = 2.50$, $a_p = 1.0$, $l_p = 15$, $R_p = 2.5$, $z/h \leq 1$)

WEIGHT = 360 LB

HORIZONTAL FORCE (E_h) = 180 W_p = 648 LB

VERTICAL FORCE (E_v) = 0.50 W_p = 180 LB

BOLT FORCES:

BOLT SPEC: 1/4"φ (A36) THREADED ROD

φT = 1599 LB/BOLT

φV = 853 LB/BOLT

TENSION (T)

$$T_u \text{ MAXIMUM} = \left[\frac{648\#(37\")(11.94\"){(0.3)}}{2 \text{ BOLTS}(23.56\")(22.75\")} \right] + \frac{648\#(37\")(13.16\"){(0.3)}}{2 \text{ BOLTS}(22.75\")(23.56\")} - \frac{(360\#)(0.9) - 180\#(11.94\")(13.16\"){(0.3)}}{2 \text{ BOLTS}(22.75\")(23.56\")} = 353 \text{ LB/BOLT (MAX)}$$

(HORIZ. - FRONT TO BACK) (HORIZ. - SIDE TO SIDE) (0.9WEIGHT) - E_v)

SHEAR (V)

$$V_u \text{ MAXIMUM} = \frac{648\#(13.16\"){(0.3)}}{4 \text{ BOLTS}(23.56\")} = 90 \text{ LB/BOLT (MAX)}$$